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# Influence of freeze-thaw cycles on the pull-out response of lime-based TRM composites

Ali Dalalbashi<sup>1</sup>, Bahman Ghiassi<sup>2</sup>, Daniel V. Oliveira<sup>3</sup>

## ABSTRACT

Textile reinforced mortars (TRMs) have emerged as a sustainable solution for strengthening existing masonry and concrete structures. As a result, many recent studies have focused on understanding the performance of these composites. However, most of these are aimed at investigating the mechanical properties of TRM composites. At the same time, their durability and long-term performance remain poorly addressed and unclear. This paper presents an experimental study on the effect of freeze-thaw environmental conditions on the micro-mechanical behavior of these composites. The results indicate that the freezing-thawing exposure conditions considered in this study do not have detrimental effects on the mortar strength. However, the fiber-to-mortar bond behavior can deteriorate because the deterioration level depends on the fiber type, embedded length, and fiber configuration.

*Keywords: Fiber/matrix bond; Pull-out test; Durability; Freeze-Thaw; Hydraulic lime mortar; TRM; FRCM; SRG*

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## 20 **1 Introduction**

21 Textile reinforced mortar (TRM) has recently raised interest among researchers and professionals  
22 as a sustainable and compatible solution for strengthening existing structures. TRM composites,  
23 also referred to as FRCM and TRC in the literature, have clear advantages when compared to fiber-  
24 reinforced polymer sheets (FRPs), such as fire resistance, sustainable and durable alternative to  
25 epoxy [1–3]. TRM composites consist of cement or lime-based mortar reinforced by steel, glass,  
26 basalt, carbon, or natural fibers. Lime-based mortars are suitable for strengthening masonry and  
27 historic structures because of their physical, chemical, and mechanical compatibility with the  
28 substrate [4,5].

29 The effectiveness of TRM strengthening systems depends on the fiber-to-mortar and TRM-to-  
30 substrate bond performance and their mechanical behavior, which have been investigated in many  
31 recent studies [6–12]. Besides, a pseudo-ductile performance of TRM composites due to the  
32 multiple cracking leads these composites to be suitable for seismic strengthening. The fiber-to-  
33 mortar bond strength provides this multiple cracking behavior [13]. Therefore understanding this  
34 mechanism and its long-term performance under harsh environmental conditions is vital for having  
35 safe and resilient strengthening solutions.

36 The durability performance of TRM composites has recently received attention from a few studies.  
37 These studies mainly focused on the tensile response, flexural behavior, and TRM-to-substrate  
38 bond performance under high-temperature, salt crystallization, and alkaline environments [14–20].  
39 Different fiber and mortar types were used to investigate the effect of freeze-thaw (FT) conditions  
40 on the mechanical behavior of TRM composites, regardless of the number of FT cycles applied to  
41 the specimens [21–26]. The FT conditions increased the ultimate tensile strength of carbon and  
42 PBO-based FRCM composites by 11-13% [24,25] and 32% [24], respectively. An earlier study  
43 [26] determined that glass-based FRCMs had a constant ultimate tensile strength after applying 40  
44 FT cycles. Meanwhile, some studies report that TRM composites exhibit a reduction in mechanical  
45 properties. For instance, the tensile and flexural strength of glass-based TRC composites decreased  
46 by 16% to 19% [21,23] and 19% [22], respectively. As a result, freeze-thaw conditions affected  
47 the mechanical performance of TRM composites differently. Additionally, in these studies, the  
48 exposed specimens were compared with control specimens at zero cycles, which can lead to an  
49 error in the analysis (especially if the matrix is a lime-based mortar). However, these studies are  
50 limited in scope, and further investigations are needed to fully understand the governing

51 deterioration mechanisms and field performance of these composites. Also, investigations on the  
52 effect of environmental conditions on the micro-mechanical response of these composites are still  
53 lacking [27].

54 This study aims to investigate the role of freeze-thaw (FT) conditions on the micro-mechanical  
55 response of TRM composites. FT conditions are chosen as one of the critical environmental  
56 conditions, especially when TRMs are applied on the outside of the buildings. Two different TRM  
57 systems (glass-based and steel-based) commonly used to repair existing masonry structures are  
58 used for the purpose of this study. The tests include mechanical characterization of the mortar and  
59 the fibers, as well as bond tests to characterize the role of embedded length and fiber configuration  
60 on the fiber-to-mortar bond behavior.

## 61 **2 Experimental program**

62 The aim is to investigate experimentally the changes in material properties and fiber-to-mortar  
63 bond behavior under FT conditions. To this end, a set of TRM composite specimens (specimen  
64 details are presented in sections 2.1 to 2.3) was prepared. After 90 days of curing in the laboratory,  
65 the specimens were exposed to zero, 60, 180, 300, and 360 FT cycles described in section 2.4 or  
66 stored in the environmental lab condition as control specimens. After that, a series of post-exposure  
67 material properties and pull-out tests (on steel and glass-based TRM composites) were conducted  
68 (see Table 1 for a detailed experimental plan and the number of specimens).

69 The pull-out specimens (which were prepared based on fiber types, bond length, and fiber  
70 configuration) are named as VWX-YZ; where V is related to the fiber type (S: steel, G: glass), and  
71 W is linked to the fiber configuration (S: single cord/yarn, T: single yarn + transverse elements,  
72 G: two cords/yarns, G': four cords). In addition, X is connected to the different embedded lengths  
73 (for glass: 50, 75, and 100 mm, for steel: 50, 150, 200, and 250 mm). Finally, Y is related to the  
74 control (C) or exposed (E) specimens, and Z represents the number of FT cycles (0, 60, 180, 300,  
75 and 360). For example, GS50-E60 is a glass-based TRM composite with a single yarn and 50 mm  
76 bond length exposed to 60 FT cycles. The materials description, sample preparation, and test  
77 methods are presented in the following sections.

### 78 **2.1 Materials**

79 All specimens were prepared using a commercial mortar, which was a hydraulic lime (NHL) and  
80 eco-pozzolan-based mortar. The reinforcing materials were a unidirectional steel mesh and woven  
3

81 biaxial glass fabric mesh. Each cord of the steel mesh was made by twisting five individual wires,  
82 with a density of 670 g/m<sup>2</sup> and an effective area of 0.538 mm<sup>2</sup>. The alkali-resistance glass fabric  
83 had a mesh size and area per unit length equal to 25×25 mm<sup>2</sup> and 35.27 mm<sup>2</sup>/ m, respectively.  
84 All specimens, including the material characterization and the pull-out tests, were cured under the  
85 same procedure until the test dates. Three days after the preparation of specimens, they were  
86 demolded. All specimens were cured under wet clothes and plastic for the first seven days, then  
87 stored and subjected to environmental lab conditions for 83 days. Earlier studies by the authors  
88 [12,27] have shown that the early maximum strength of the lime-based mortar occurs after 90 days.  
89 Then, one part of the specimens was exposed to the planned FT cycles, while the remaining  
90 specimens were kept in the lab as control specimens and tested simultaneously with the exposed  
91 specimens (see Fig. 1 and Table 1).

## 92 2.2 Materials characterization test

93 Differential thermal analyses (DTA) were conducted with a Q600 TA Instrument apparatus to  
94 quantify and identify chemical composition by observing the thermal behavior of mortar samples  
95 after different exposure times. The mortar samples were heated from 50°C to 1000°C at a rate of  
96 10°C/min in an aluminum pan and under 100 ml/min of N<sub>2</sub> flow. All DTA samples (25 mg) were  
97 extracted from the pull-out specimens at the bond interface after testing (for each series, three  
98 samples were prepared).

99 The compressive strength of mortar was measured by performing cubic specimens  
100 (50×50×50 mm<sup>3</sup>) and following ASTM C109 [28]. The tests were performed under force-  
101 controlled conditions, at a rate of 10 N/s, and using a Lloyd testing machine, as shown in Fig. 2a.  
102 The flexural strength of the mortar was tested according to EN 1015-11 [29], where specimens  
103 had a prismatic shape (40×40×160 mm<sup>3</sup>). Three-point bending tests were conducted for this  
104 purpose with a Lloyd testing machine under force-controlled conditions at a rate of 150 N/s (Fig.  
105 2b). Additionally, the elastic modulus of mortar was characterized according to EN 12390-13 [30]  
106 using cylinder-shaped specimens (150 mm in length and 70 mm diameter) and using a universal  
107 testing machine (load capacity of 100 kN). Three LVDTs with a 5 mm range and 1-μm sensitivity  
108 were mounted on the specimens to measure the deformations of specimens (see Fig. 2c).

109 The fabric tensile strength was characterized by performing direct tensile tests on the single steel  
110 cords and single glass yarns (warp direction). A universal testing machine (load capacity of 10 kN)

111 under displacement-controlled conditions (0.3 mm/min) applied tensile loads to the samples. A  
112 100 mm clip gauge located at the center of the specimens was used to measure the deformations  
113 during the tests, as shown in Fig. 2d.

### 114 2.3 Pull-out tests

115 A single-sided pull-out test setup developed in [31] was used to investigate the fiber-to-mortar  
116 bond behavior. The samples were prepared following the methodology explained in [31] and the  
117 geometry shown in Fig. 3a. Preparation of pull-out specimens firstly involved embedding the fiber  
118 in an epoxy resin block in 200 mm length. The pull-out specimens had a flat disk-shaped mortar  
119 with a width of 125 mm and a thickness of 16 mm. Pull-out specimens were prepared by applying  
120 8 mm of mortar to the inside of the wood mold, placing fibers, and then applying another layer of  
121 mortar with 8 mm thick. To evaluate the role of the fabric architecture, different configurations  
122 were considered for each type of fiber. In the steel-based TRM, which was composed of a  
123 unidirectional steel mesh, this consisted of evaluating the role of the cords number ("single cord,"  
124 "two cords," and "four cords") and embedded length (50, 150, 200, and 250 mm), as presented in  
125 Fig. 3b. It should be mentioned that samples were prepared with only 150 mm embedded length.  
126 Since the glass fabric was a bidirectional mesh, the samples consisted of: "single (warp) yarn,"  
127 "single yarn + transverse elements," and "group of yarns" (Fig. 3c). The bond length of the "single  
128 yarn" specimens was 50, 75, and 100 mm, while the bond length of other groups was 50 mm. The  
129 considered bond length for the steel and glass-based TRMs was determined based on the  
130 experimental results reported by the authors in [32]. In the "single yarn + transverse elements"  
131 specimens, two transverse elements (weft yarns) were embedded in the mortar with a total length  
132 of 25 mm, 12.5 mm at each side, equal to half of the mesh opening. In the "group of yarns," two  
133 weft yarns were embedded in the mortar, in which their total lengths were equal to 50 mm  
134 (12.5 mm at each side and 25 mm distance between warp yarns), as shown in Fig. 3c.  
135 For performing the pull-out tests, two U-shape steel supports fixed the specimens (see Fig. 3d). A  
136 servo-hydraulic system (load capacity of 25 kN) applied tensile loads to the epoxy resin block  
137 from the top with a mechanical clamp under displacement-controlled conditions and at a rate of  
138 1.0 mm/min. Three LVDTs with a 20 mm range and 2- $\mu$ m sensitivity measured the fiber-to-mortar  
139 slip. LVDT average measurements are shown as slip in the results of the experiment.

## 140 2.4 Environmental conditions

141 A Fitoclima 6400 EC25 climatic chamber was used to expose the samples to freeze-thaw (FT)  
142 conditions. The FT cycles consisted of thawing the samples at 30°C and 90% RH for two hours;  
143 then, the temperature was decreased at a 0.111°C/min rate until reaching -10°C. After this point,  
144 samples were frozen for two hours, followed by a temperature increase at a rate of 0.111°C/min  
145 till reaching 30°C. Fig. 4a shows the planned and the real conditions (including temperature and  
146 humidity) inside the climatic chamber, where the real temperature and humidity were measured  
147 by the average of two standalone data loggers.

148 This cycle of sixteen hours was repeated 360 times. The selected FT environmental condition  
149 aimed to create an environment in the laboratory to represent real environmental conditions in an  
150 accelerated way. A similar FT condition was also performed in another study [33] to investigate  
151 the performance of FRP strengthening systems. Several control samples were also prepared and  
152 placed in the laboratory in parallel to the FT tests. The environmental conditions of the storage  
153 laboratory during this period are presented in Fig. 4b (average temperature and humidity were  
154 18°C and 75% RH).

155 Five specimens from each series of tests (material characterization and pull-out tests) were taken  
156 from the climatic chamber room (at 20°C) per 60 cycles (equal to 40 days), as presented in Fig. 1.  
157 They were then placed in the laboratory conditions for seven days before performing the post-  
158 exposure tests.

## 159 **3 Results and discussion**

### 160 3.1 Material properties

161 DTA analysis graph shows three peaks (Fig. 5) attributed to hydrated water (50-100°C),  
162 dehydroxylation (losing bound water at 380-400°C), and decarboxylation (releasing CO<sub>2</sub> at 680-  
163 780°C) [34–37]. DTA results show that decarboxylation increases with time under both the control  
164 and the FT conditions due to the large amount of CaCO<sub>3</sub> when it is compared to the control  
165 specimens at 90 days of age (C0). The dehydroxylation and decarboxylation changes obtained  
166 from the DTA test show that the used lime-based mortar is still hardening at older ages under both  
167 the control and the FT conditions. Generally, hydraulic lime-based mortar hardens with a  
168 combination of hydration and carbonation, according to [38]. Therefore, from these outputs and

169 previous results [27], it can be concluded that using a hydraulic lime-based mortar at an early age  
170 for durability tests can lead to erroneous results.

171 Table 2 reports the changes in the strength of mortar and fibers under both the control and the FT  
172 conditions. The mortar compressive strength remains constant until the end of tests under both  
173 conditions. However, by ignoring the variation of the results, the mortar compressive strength  
174 shows a slight increment. This increment is 3% in the control samples (C360), and 12% in the FT  
175 exposed samples (E360) when compared to the control specimens at zero cycles (C0). Similar  
176 behavior is also observed for the flexural strength and the elastic modulus. These observations  
177 show that the considered FT conditions do not have a detrimental effect on the mortar strength but  
178 lead to a slight enhancement of properties, possibly by promoting mortar hydration under high  
179 humidity conditions, which is in line with the DTA results. Besides, results are consistent with the  
180 changes in mortar strength under indoor conditions reported in [27]. As for the fibers, glass yarns  
181 do not show any deterioration, but the tensile strength of steel cords and elastic modulus under the  
182 FT conditions declined slightly by 5% and 9%, respectively.

### 183 3.2 Effect of embedded length on the pull-out response

#### 184 3.2.1 *Steel-reinforced mortar*

185 Fig. 6 shows the average load-slip curves of steel-based TRM with different embedded lengths  
186 under both the control and the FT conditions. Table A 1 also presents the failure mode of these  
187 specimens. All SS50 and SS150 specimens show a fiber slipping/pull-out failure mode with the  
188 typical load-slip curves, including the linear, nonlinear, and dynamic stages. The linear stage  
189 exhibits a complete bond between fiber and mortar, while the nonlinear stage indicates debonding  
190 occurring at the fiber-to-mortar interface and continues until the peak load. Then, complete  
191 debonding occurs, and the fiber pulls out from the mortar (the dynamic stage). For more  
192 information related to the pull-out mechanism, the reader is referred to [31,39]. The failure mode  
193 in SS200 is a combination of the fiber rupturing (for SS200-E60, SS200-E180) and slipping (for  
194 SS200-E300/360 and SS200-C0/360), as listed in Table A 1. The load-slip curves of the specimens  
195 with the fiber rupture show a linear and a partially nonlinear part until the peak load, followed by  
196 a sudden rupture of the fiber (Fig. 6). The failure of SS250 specimens is the fiber rupture (for  
197 SS250-C0 and SS250-E360) and fiber slipping (for SS250-E60/180/300 and SS250-C360) (see  
198 Table A 1). The fiber rupture occurs at the loaded end (inside the mortar or at the mortar interface)



199 due to reaching the applied load to the fiber tensile strength. Fiber rupturing shows that the bond  
200 strength at the interface of the fiber-to-mortar was higher than the tensile strength of the steel fibers  
201 and caused fiber failure. In Table A 1, the peak load of all steel-based TRM specimens with fiber  
202 failure is close to the tensile strength of the steel fiber (2819 MPa or 1517 N, as listed in Table 2).  
203 In general, in all embedded lengths, the control samples show a deterioration of bond performance  
204 with time (comparing the load-slip curves of C0 with C360 in Fig. 6). This observation is in line  
205 with that reported in [27]. The FT exposure, however, has different effects on specimens with  
206 different embedded lengths. For example, the load-slip curves of SS50-E and SS200-E specimens  
207 get flattered by increasing the number of cycles. In contrast, SS150-E and SS250-E show the  
208 opposite trend. Additionally, a few load-slip curves of steel-based TRM specimens (SS50-C360,  
209 SS150-C360, and SS200-E360) show a load drop after reaching peak load, followed by a slip-  
210 hardening effect (see Fig. 6). The fiber-to-mortar bond in a TRM composite with a high adhesion  
211 must be broken before the dynamic stage can begin. The load drop occurs when the load required  
212 to debond the fiber is higher than the frictional resistance after complete debonding, resulting in  
213 an unstable debonding [40]. In addition, slip hardening occurs due to increasing friction stress  
214 between the fiber and the mortar at the dynamic stage, as reported in [39].

215 The changes of the peak load ( $P_P$ ), the debonding energy ( $E_{deb}$ ), and the pull-out energy ( $E_{po}$ ) with  
216 exposure are presented in Fig. 7 (the average values are also reported in Table A 1).  $E_{deb}$  and  $E_{po}$   
217 are defined as the area under the load-slip curve until the peak load and from the peak load until  
218 the end, respectively. It should be mentioned that  $E_{po}$  is not calculated for SS250-E360 and SS250-  
219 C0 as the fibers failed at the peak load (see Fig. 6d). All pull-out parameters show, in general, a  
220 gradual decrement from 0 to 360 cycles under both the control and the FT conditions, as shown in  
221 Fig. 7. To better understand the effect of mortar age and FT conditions on the bond parameters,  
222 the difference (in percentage) between the average results at 360 cycles (C360 and E360) and the  
223 control specimens at 0 cycles (C0) are presented in Table A 1. It can be inferred that the bond  
224 parameters are deteriorated equally under both conditions, showing the proposed FT condition was  
225 not harsh enough. It seems other parameters cause the bond degradation to occur in both  
226 conditions, such as the long-term shrinkage effect by forming micro cracks at the bond interface.  
227 Continuing hydration (as mentioned in section 3.1) may lead to chemical shrinkage due to a  
228 reduction in the hydration volume of anhydrous compounds [41]. This output should be further  
229 investigated in future studies.

### 230 3.2.2 *Glass-reinforced mortar*

231 The average of pull-out response curves obtained for the glass-based TRMs with different bond  
232 lengths is presented in Fig. 8. Besides, Table A 2 presents the failure of these specimens. The load-  
233 slip curves of GS50 specimens includes the linear and nonlinear part until reaching the first peak  
234 load, followed by slip hardening effect and then decreasing load. These specimens generally fail  
235 under yarn slipping/pull-out mode though tensile rupture of the yarns occurs in GS50-E60/180 and  
236 GS50-C0 specimens at the dynamic stage. The failure of GS75 specimens is yarn slipping followed  
237 by rupturing. This observation is also supported by their load-slip curves, where the yarn slipped  
238 until reaching peak load and then ruptured at the dynamic stage. Meanwhile, GS100 samples are  
239 failed by fiber rupturing, so their load-slip curves only include the linear and nonlinear parts until  
240 a peak load is reached.

241 Load-slip curves show a decrease in bond performance of GS50 specimens. So, as the FT cycle or  
242 mortar age increases, the load-slip curve of GS50 specimens becomes flatter and the slip hardening  
243 effect is reduced due to the decrease in the friction stress at the bond interface. In contrast, other  
244 glass-based TRMs (GS75 and GS100) appear to show better performance with an increase in  
245 mortar age, both under the control and the FT conditions. This observation finds that both  
246 conditions have an adverse effect on the glass-based TRM when the bond length is equal to or less  
247 than the effective bond length (50 mm based on [32]), where the load slip curve is flattened by  
248 increasing the number of cycles. However, the development length in tension does not decrease  
249 for longer embedded lengths (75 and 100 mm), as the load-slip curves increase after 360 FT cycles.  
250 Fig. 9 shows the key parameters of the individual pull-out specimens with the regression line to  
251 show the general behavior of the glass-based TRM under the FT conditions. Since tensile rupture  
252 occurs at the peak load in GS75 and GS100 specimens,  $E_{po}$  is not presented for these specimens.  
253 Also, Table A 2 presents the difference between the FT and the control samples after 360 cycles  
254 with respect to the control conditions (C0), for a better analysis. The results show that the pull-out  
255 parameters of the GS50 specimens decrease under both conditions, compared to the GS75 and  
256 GS100 specimens showing an increase in the bond parameters. Due to the same decrease in pull-  
257 out parameters of GS50 specimens under both conditions, the proposed FT condition does not  
258 affect the bond behavior. Instead, it seems that a sort of bond deterioration by forming micro-  
259 cracks at the bond interface has occurred. One possible explanation is the negative impact of mortar  
260 hydration on the bond behavior of GS50 specimens, which continues until the end of the tests at

261 both conditions (see section 3.1). This negative effect can manifest in the form of chemical  
262 shrinkage or notching of the yarn surface due to the formation of precipitates [23]. Future studies  
263 need to explore this output more thoroughly. On the other hand, the mortar hydration does not  
264 affect the pull-out parameters of GS75 and GS100 specimens, which can be due to the longer  
265 embedded length of these specimens than GS50 specimens.

### 266 3.2.3 Comparison between steel and glass-based TRMs

267 To compare the behavior of steel and glass-based TRM under FT conditions, the results of SS50-  
268 E and GS50-E with equal bond lengths are selected. Generally, the steel-based TRMs (SS50-E)  
269 show a better performance than the glass-based TRMs (GS50-E) under the considered FT  
270 conditions. Comparing the load-slip curves of GS50 and SS50 specimens confirms this  
271 observation. In this way, glass-based TRMs show wide curves at the beginning of exposure, and  
272 by increasing the number of FT cycles, they become narrow and flat. On the other hand, the steel-  
273 based TRMs show wide curves at all cycles and only decrease at the end of the exposure. Besides,  
274 Fig. 10 compares the pull-out parameters of SS50-E and GS50-E specimens, in which standard  
275 deviations are presented by the error bar. The results show that the peak load ( $P_P$ ) and the pull-out  
276 energy ( $E_{po}$ ) of both systems are approximately equal (by considering the error bar). However, the  
277 debonding energy ( $E_{deb}$ ) of SS50-E is higher than the GS50-E one due to the different transitions  
278 between the nonlinear and dynamic stages at these specimens.

## 279 3.3 Effect of textile configuration

### 280 3.3.1 Steel-reinforced mortar

281 The failure mode of SG150 specimens (with two cords) is generally fiber rupturing under the  
282 control and the FT conditions; however, SG150-C0 specimens show fiber slipping, as presented  
283 in Table A 3. Also, SG'150 specimens (with four cords) fail due to fiber slipping/pull-out under  
284 both conditions. The results also show that the failure modes of SG150 and SG'150 do not change  
285 from the control to the exposed specimens for the suggested period, like the single steel fiber  
286 specimens (SS150). The average of load-slip curves of both SG150 and SG'150 specimens are  
287 presented in Fig. 11. The pull-out response of SG150 specimens includes the linear and nonlinear  
288 stages, and by reaching the peak load, the load drops suddenly due to the fiber rupturing. On the  
289 other hand, the load-slip curves of SG'150 specimens show a typical pull-out curve.

290 Compared to SS150 specimens, the pull-out response of SG'150 samples (with four cords) shows  
291 the load decreasing with a steep slope after peak load under the control and the FT conditions.  
292 Increasing the number of fibers results in a decrease in the load carried by each fiber, which is due  
293 to the fiber volume fraction effect [42]. In addition, the pull-out response of the SG'150-C and  
294 SG'150-E specimens decreases by increasing the mortar age or increasing the number of the FT  
295 cycles, as shown in Fig. 11. In contrast to SS150 and SG'150, there are no changes from the load-  
296 slip curve of the SG150-C to SG150-E specimens.

297 The pull-out parameters of the individual specimens under the control and the FT conditions are  
298 reported in in Fig. 12 and their average values are presented in Table A 3. Since tensile rupture  
299 occurs at the peak load in SG150 specimens,  $E_{po}$  is not presented for these specimens. The results  
300 display that the FT condition causes the pull-out parameters to decrease slightly in SG150-E and  
301 SG'150-E specimens (although the  $P_P$  of the SG150-E increases slightly). However, in group fibers  
302 (SG150 and SG'150), bond parameters decrease less than those in single fibers (SS150) under  
303 freeze-thaw conditions. The difference of the bond parameters between the freezing-thawing  
304 exposure (E360) and the control specimens (C0), as well as between C360 and C0 specimens,  
305 shows that the FT condition does not affect the bond parameters of SG150 and SG'150 samples,  
306 see Table A 3. Again, the effects of chemical shrinkage on bond degradation can be emphasized  
307 further here due to continuing mortar hydration, as shown in section 3.1.

### 308 3.3.2 *Glass-reinforced mortar*

309 Fig. 13 shows the average load-slip curves of GT50 (with transverse yarns) and GG50 (2 group  
310 yarns) specimens under both the control and the FT conditions. In addition, Table A 4 reports their  
311 failure mode. All GT50 specimens fail under yarn slipping/pull-out followed by rupturing. This  
312 observation is supported by their load-slip curves, including the linear, nonlinear, and partially  
313 dynamic stages. A similar failure mode also is observed for all GG50 specimens, except GG50-  
314 C360 failed by tensile rupture when the peak load was reached. A comparison among the load-slip  
315 curves of GT50, GG50, and GS50 illustrates the positive effect of transverse elements, so the pull-  
316 out curves of GT50 and GG50 do not show load decreasing after peak load (dynamic stage). The  
317 load-slip curves of the control specimens (GT50-C and GG50-C) show that the pull-out response  
318 improves by increasing the mortar age, in contrast with the GS50-C specimens. Like GS50-E, the  
319 FT condition slightly declines the load-slip curves of GT50-E and GG50-E.

320 The key characteristics of the pull-out response of the individual GT50 and GG50 specimens are  
321 presented in Fig. 14 (see Table A 4 for the average of the pull-out parameters at each cycle). Since  
322 tensile rupture occurs at the peak load in several GG50 specimens,  $E_{po}$  is not presented for these  
323 specimens in Fig. 14c. Under the control condition,  $P_P$  of the GT50-C and GG50-C shows an  
324 incremental trend by increasing the mortar age though other pull-out parameters decline. Besides,  
325 a comparison between the GT50-C and GG50-C shows that the key characteristics of GG50-C  
326 specimens are higher than the pull-out parameters of GT50-C specimens. This observation reveals  
327 that fabric mesh influences the yarn-to-mortar bond behavior more than the single yarn with the  
328 transverse elements, even at different mortar ages.

329 Similar to the single glass-based TRM (GS50), the FT condition decreases the pull-out parameters  
330 in GG50-E, as shown in Fig. 14. However, GT50 specimens show an increasing trend under the  
331 FT conditions. Table A 4 also presents the difference of the bond parameters between the freezing-  
332 thawing exposure (E360) and the control (C0) specimens, as well as C360 and C0 specimens. The  
333 outcomes illustrate that under both conditions, the bond properties of GT50 specimens improve.  
334 Meanwhile, the outcomes display that the FT conditions lead to a considerable decrement of all  
335 pull-out parameters in GG50-E360 specimens, in contrast to GG50-C360. As a result of this  
336 observation, it is apparent that glass fabric configuration affects the pull-out response, resulting in  
337 different bond behavior under FT conditions. Therefore, studying from single to mesh  
338 configurations of this type of fiber is crucial to better understand their behavior.

#### 339 **4 Conclusions**

340 The effect of freeze-thaw (FT) conditions on the micro-mechanical response of steel and glass-  
341 based TRM composites was examined in this study. This research included investigating the  
342 mortar strength changes and the bond performance as a function of embedded length, number of  
343 yarns/cords, use of transverse fibers, and age. In general, the following conclusions can be drawn:

- 344 • The mechanical properties of the lime-based mortar used in this study remained constant  
345 (with a slight improvement) under both the control and FT conditions. This observation  
346 shows that the detrimental effect of the considered FT conditions was less than the mortar  
347 hydration positive effect on the strength. This can also be due to the fact that although 90%  
348 RH was considered in the FT exposure conditions, this condition might not have been

349 sufficiently high for saturating the samples until a critical level in the time frame of the  
350 experimental tests.

- 351 • As expected, the mechanical properties of the glass fibers were not affected by the FT  
352 conditions. However, the tensile strength of steel fibers decreased slightly.
- 353 • The pull-out response of the steel-based TRMs with different bond lengths generally  
354 deteriorated both in the control and under the FT conditions. The highest deterioration was  
355 observed in the 50 mm bonded length samples under FT conditions. However, the bond  
356 deterioration trend decreased by increasing the bond length. This was expected as with an  
357 increment of the bond length, the importance of local bond deterioration becomes less  
358 significant.
- 359 • The effect of FT conditions on the glass-based TRM varied with different embedded lengths.  
360 While 50 mm embedded length samples showed deterioration of the bond strength, 75 mm  
361 and 100 mm samples showed an enhancement of the bond strength (peak load and debonding  
362 energy). A similar observation was also found for the specimens cured in the lab conditions.
- 363 • The effect of FT conditions on the pull-out response of the group steel-based TRM was  
364 significantly different than that of the single cord samples. While both control and FT  
365 conditions led to enhancement of the peak load in the samples reinforced with two cords,  
366 those conditions caused deterioration in single and four cord samples. While this requires  
367 further investigations, it shows the importance of considering the group behavior in closely  
368 distanced fabrics.
- 369 • The yarn configuration was also found to be important in glass-based TRMs (in which a  
370 bidirectional fabric was used). While the bond behavior of the specimens with transverse  
371 yarns enhanced under the FT conditions, the samples with group yarns showed a  
372 considerable deterioration similar to those observed in single yarn samples.

373 These observations show the importance of considering the actual architecture of the fabrics in  
374 experimental specimens for evaluating the mechanical and durability performance of TRM  
375 composites. Also, consideration of other FT exposure conditions is suggested to be considered in  
376 future studies.

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494 **6 Appendix**

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Table A 1. Changes of bond properties in steel-based TRM with different embedded lengths\*.

Embedded length [mm]	Name	P <sub>p</sub> [N]	P <sub>p</sub> /P <sub>p,C0-1</sub> [%]	E <sub>deb</sub> [N.mm]	E <sub>deb</sub> /E <sub>deb,C0-1</sub> [%]	E <sub>po</sub> [N.mm]	E <sub>po</sub> /E <sub>po,C0-1</sub> [%]	Failure	Number of specimens
50	SS50-C0	587.9 (14)	-	1545.8 (17)	-	4209.9 (14)	-	slip	4
	SS50-E60	412.5 (13)	-	1197.3 (38)	-	2495.4 (15)	-	slip	4
	SS50-E180	584.1 (22)	-	1102 (34)	-	4314 (23)	-	slip	5
	SS50-E300	349.8 (29)	-	798.8 (30)	-	3110 (30)	-	slip	5
	SS50-E360	420.3 (22)	-29	1168.6 (49)	-24	2859.5 (16)	-32	slip	5
	SS50-C360	535.6 (15)	-9	99.1 (30)	-94	5445.5 (14)	29	slip	4
150	SS150-C0	1279.7 (11)	-	3810.1 (25)	-	9816 (13)	-	slip	5
	SS150-E60	1301.3 (22)	-	2892.6 (11)	-	6755.2 (15)	-	slip	5
	SS150-E180	1032.6 (26)	-	1444.8 (39)	-	8503.1 (23)	-	slip	5
	SS150-E300	1087.4 (18)	-	2578.7 (25)	-	8186.3 (18)	-	slip	5
	SS150-E360	1389.4 (7)	9	3772.8 (42)	-1	10883.5 (25)	11	slip	4
	SS150-C360	874.6 (19)	-32	562.5 (40)	-85	9698.4 (10)	-1	slip	5
200	SS200-C0	1164.2 (26)	-	4033.4 (39)	-	7747.7 (21)	-	slip	5
	SS200-E60	1622.8 (2)	-	2051 (15)	-	-	-	rupture	4
	SS200-E180	1622.2 (1)	-	1994.8 (16)	-	-	-	rupture	5
	SS200-E300	1289.4 (13)	-	3759.2 (22)	-	9540.4 (13)	-	slip	5
	SS200-E360	980.9 (11)	-16	1021.1 (25)	-75	8616.4 (6)	11	slip	4
	SS200-C360	923.3 (12)	-21	3140.4 (38)	-22	7374 (14)	-5	slip	5
250	SS250-C0	1642.5 (3)	-	3874.1 (24)	-	-	-	rupture	4
	SS250-E60	1568.1 (7)	-	3742.7 (43)	-	11200.6 (2)	-	slip	5
	SS250-E180	1526.7 (4)	-	2798.7 (13)	-	12797.3 (9)	-	slip	5
	SS250-E300	1166.9 (11)	-	3446.9 (12)	-	8481.2 (3)	-	slip	5
	SS250-E360	1690.2 (2)	3	2456.4 (24)	-37	-	-	rupture	5
	SS250-C360	1507.3 (10)	-8	4541.4 (16)	17	11538.9 (7)	-	slip	5

497 \*Coefficients of variation in percentage are provided inside parentheses.

498 P<sub>p</sub>: Peak load; E<sub>deb</sub>: Debonding energy; E<sub>po</sub>: Pull-out energy.

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Table A 2. Changes of bond properties in glass-based TRM with different embedded lengths\*.

Embedded length [mm]	Name	P <sub>p</sub> [N]	P <sub>p</sub> /P <sub>p,C0-1</sub> [%]	E <sub>deb</sub> [N.mm]	E <sub>deb</sub> /E <sub>deb,C0-1</sub> [%]	E <sub>po</sub> [N.mm]	E <sub>po</sub> /E <sub>po,C0-1</sub> [%]	Failure	Number of specimens
50	GS50-C0	502.3 (14)	-	208.1 (9)	-	4161.9 (34)	-	slip	4
	GS50-E60	513.5 (6)	-	105.6 (25)	-	4286.1 (27)	-	slip	4
	GS50-E180	502.4 (14)	-	191.4 (26)	-	4339.0 (3)	-	slip	4
	GS50-E300	329.2 (11)	-	90.6 (24)	-	3748.3 (26)	-	slip	4
	GS50-E360	308.0 (10)	-39	31.5 (15)	-85	2573.8 (13)	-38	slip	4
	GS50-C360	308.4 (24)	-39	63.3 (25)	-70	2776.0 (25)	-33	slip	4
75	GS75-C0	613.1 (6)	-	301.7 (17)	-	-	-	slip followed by rupture	5
	GS75-E60	569.3 (18)	-	258.3 (37)	-	-	-	slip followed by rupture	4
	GS75-E180	545.4 (14)	-	185.3 (27)	-	-	-	slip followed by rupture	4
	GS75-E300	576.8 (10)	-	171.1 (18)	-	-	-	slip followed by rupture	5
	GS75-E360	724.4 (4)	18	1206 (14)	300	-	-	slip followed by rupture	5
	GS75-C360	695.6 (17)	13	294.8 (32)	-2	-	-	slip followed by rupture	5
100	GS100-C0	722.5 (7)	-	592.9 (32)	-	-	-	rupture	5
	GS100-E60	827.9 (9)	-	926.3 (36)	-	-	-	rupture	5
	GS100-E180	701.5 (4)	-	482.5 (15)	-	-	-	rupture	4
	GS100-E300	830.7 (8)	-	699.8 (16)	-	-	-	rupture	5
	GS100-E360	871.3 (7)	21	785.5 (19)	32	-	-	rupture	5
	GS100-C360	840.9 (6)	16	605.3 (17)	2	-	-	rupture	5

504 \*Coefficients of variation in percentage are provided inside parentheses.

505 P<sub>p</sub>: Peak load; E<sub>deb</sub>: Debonding energy; E<sub>po</sub>: Pull-out energy.

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Table A 3. Changes of bond properties in steel-based TRM with different fiber configurations\*.

Fiber configuration	Name	P <sub>p</sub> [N]	P <sub>p</sub> /P <sub>p,C0-1</sub> [%]	E <sub>deb</sub> [N.mm]	E <sub>deb</sub> /E <sub>deb,C0-1</sub> [%]	E <sub>po</sub> [N.mm]	E <sub>po</sub> /E <sub>po,C0-1</sub> [%]	Failure	Number of specimens
Two cords	SG150-C0	1489.2 (11)	-	2253 (37)	-	-	-	slip	5
	SG150-E60	1494.3 (5)	-	1915 (38)	-	-	-	rupture	5
	SG150-E180	1625.1 (1)	-	1901.9 (15)	-	-	-	rupture	5
	SG150-E300	1630.7 (5)	-	1919.4 (16)	-	-	-	rupture	5
	SG150-E360	1627.2 (5)	9	1832.4 (17)	-19	-	-	rupture	5
	SG150-C360	1536.1 (9)	3	1446.8 (15)	-36	-	-	rupture	4
Four cords	SG'150-C0	1304 (5)	-	1700 (19)	-	6441.7 (21)	-	slip	5
	SG'150-E60	1224.7 (5)	-	2019.1 (18)	-	7175.8 (12)	-	slip	5
	SG'150-E180	1424.6 (6)	-	1721 (15)	-	6279.6 (6)	-	slip	5
	SG'150-E300	1364.2 (7)	-	1883.8 (18)	-	7683 (4)	-	slip	5
	SG'150-E360	1231.3 (7)	-6	1918.2 (23)	13	6868.3 (17)	7	slip	5
	SG'150-C360	847.7 (14)	-35	1435.3 (16)	-16	5998.9 (8)	-7	slip	5

\*Coefficients of variation in percentage are provided inside parentheses.

P<sub>p</sub>: Peak load; E<sub>deb</sub>: Debonding energy; E<sub>po</sub>: Pull-out energy.

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Table A 4. Changes of bond properties in glass-based TRM with different fiber configurations\*.

Fiber configuration	Name	P <sub>p</sub> [N]	P <sub>p</sub> /P <sub>p,C0-1</sub> [%]	E <sub>deb</sub> [N.mm]	E <sub>deb</sub> /E <sub>deb,C0-1</sub> [%]	E <sub>po</sub> [N.mm]	E <sub>po</sub> /E <sub>po,C0-1</sub> [%]	Failure	Number of specimens
Single yarn+ transverse elements	GT50-C0	272.9 (17)	-	28.3 (35)	-	4026.1 (42)	-	slip	5
	GT50-E60	716.6 (10)	-	855.4 (56)	-	2794.5 (37)	-	slip	5
	GT50-E180	572.7 (11)	-	1041.1 (39)	-	-	-	slip	5
	GT50-E300	458.6 (17)	-	124 (27)	-	2352.5 (38)	-	slip	5
	GT50-E360	456.2 (12)	67	80.2 (22)	183	4733.5 (14)	18	slip	4
	GT50-C360	449.1 (26)	65	82.2 (59)	190	1771.1 (71)	-56	slip	4
Two group yarns	GG50-C0	641.9 (8)	-	2252.4 (34)	-	-	-	slip	5
	GG50-E60	605.1 (6)	-	3362.7 (12)	-	-	-	slip	5
	GG50-E180	383 (21)	-	71.0 (51)	-	2073 (44)	-	slip	5
	GG50-E300	368.1 (3)	-	44.0 (21)	-	5268.2 (6)	-	slip	4
	GG50-E360	401.0 (11)	-38	89.5 (30)	-96	5091.1 (18)	-	slip	5
	GG50-C360	796.0 (13)	24	1084.5 (30)	-52	-	-	rupture	5

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\*Coefficients of variation in percentage are provided inside parentheses.

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P<sub>p</sub>: Peak load; E<sub>deb</sub>: Debonding energy; E<sub>po</sub>: Pull-out energy.

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Table 1. Experimental program.

Test	material	Fiber configuration	Bond length [mm]	Freeze-thaw cycles					Name	Number of specimens × cycles
				0	60	180	300	360		
Compressive	Mortar	-	-			*		*	-	5 × 7
Flexural	Mortar	-	-			*		*	-	5 × 5
Elastic modulus	Mortar	-	-			*		*	-	5 × 5
Tensile	Glass and steel fibers	-	-					*	-	5 × 3
Pull-out (embedded length)	Glass yarn and mortar	Single yarn	50					*	GS50-C0, GS50-C360, GS50-E60~GS50-E360	5 × 7
			75					*	GS75-C0, GS75-C360, GS75-E60~GS75-E360	5 × 7
			100					*	GS100-C0, GS100-C360, GS100-E60~GS100-E360	5 × 7
	Steel fiber and mortar	Single cord	50					*	SS50-C0, SS50-C360, SS50-E60~SS50-E360	5 × 7
			150					*	SS150-C0, SS150-C360, SS150-E60~SS150-E360	5 × 7
			200					*	SS200-C0, SS200-C360, SS200-E60~SS200-E360	5 × 7
			250					*	SS250-C0, SS250-C360, SS250-E60~SS250-E360	5 × 7
Pull-out (fiber configuration)	Glass yarn and mortar	Single yarn + transverse	50					*	GT50-C0, GT50-C360, GT50-E60~GT50-E360	5 × 7
		Group (2 yarns)						*	GG50-C0, GG50-C360, GG50-E60~GG50-E360	5 × 7
	Steel fiber and mortar	Group (2 cords)	150					*	SG150-C0, SG150-C360, SG150-E60~SG150-E360	5 × 7
		Group (4 cords)						*	SG'150-C0, SG'150-C360, SG'150-E60~SG'150-E360	5 × 7

542 \*Tested both control and exposed specimens; the grey cells indicate the performed tests.

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Table 2. Mortar and fibers Mechanical properties\*.

Strength [MPa]	Material	Control specimens corresponding to FT exposures cycles			Exposed specimens			
		C0	C180	C360	E60	E180	E300	E360
Compressive strength	Mortar	16.8 (11)	20 (12)	17.3 (10)	17.0 (10)	19.5 (5)	17.3 (2)	18.8 (3)
Flexural strength	Mortar	4.5 (2)	4.5 (12)	4.7 (5)	-	5.8 (5)	-	5.0 (5)
Elastic modulus	Mortar	6713 (6)	8280 (11)	8095 (10)	-	7593 (1)	-	7462 (12)
	Steel fiber	189340 (8)	-	-	-	-	-	173000 (2)
	Glass fiber	65940 (5)	-	-	-	-	-	70720 (3)
Tensile strength	Steel fiber	2972 (8)	-	-	-	-	-	2819 (1)
	Glass fiber	875 (13)	-	-	-	-	-	899 (5)

545 \*Coefficient of variation of the results is given in percentage inside parentheses.

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562 (b) debonding energy; (c) pull-out energy.

563 Fig. 11. Pull-out response of steel-based TRMs with different configurations: (a) SG150; (b) SG'150.

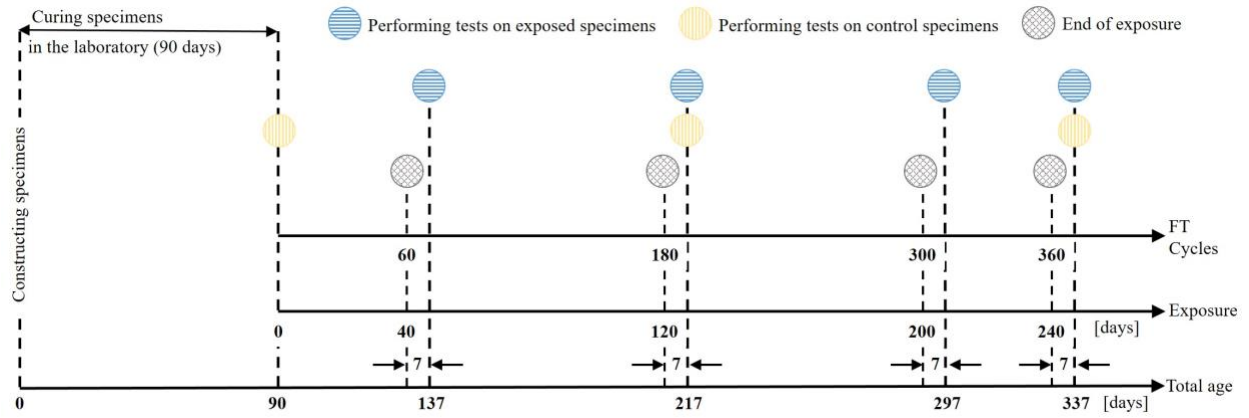
564 Fig. 12. Pull-out parameters of group steel-based TRM: (a) peak load; (b) debonding energy; (c) pull-out energy.

565 Fig. 13. Pull-out response of glass-based TRMs with different configurations: (a) GT50; (b) GG50.

566 Fig. 14. Pull-out parameters of single+ transverse and group glass-based TRM: (a) peak load; (b) debonding energy;  
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Fig. 1. Schematic representation of the test program.





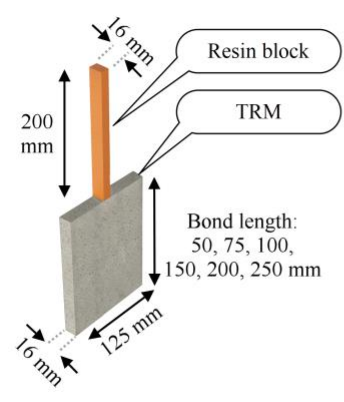
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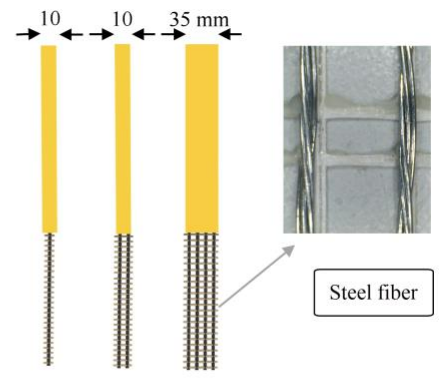
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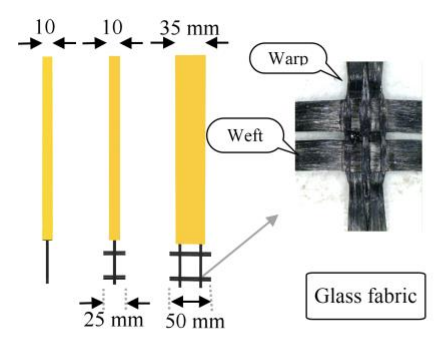
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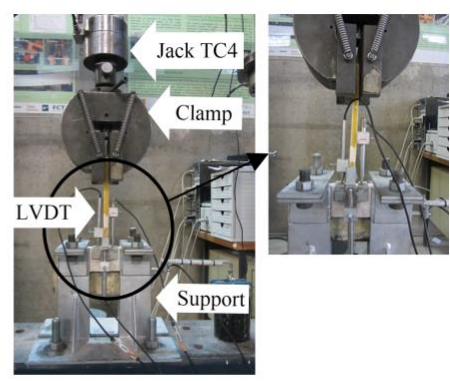
(a)



(b)



(c)



(d)

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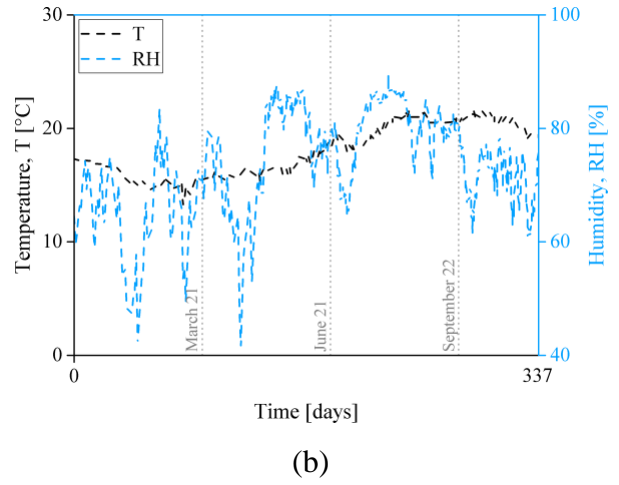
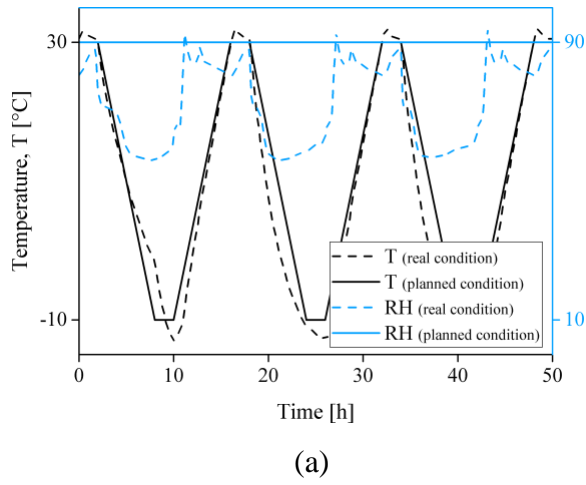
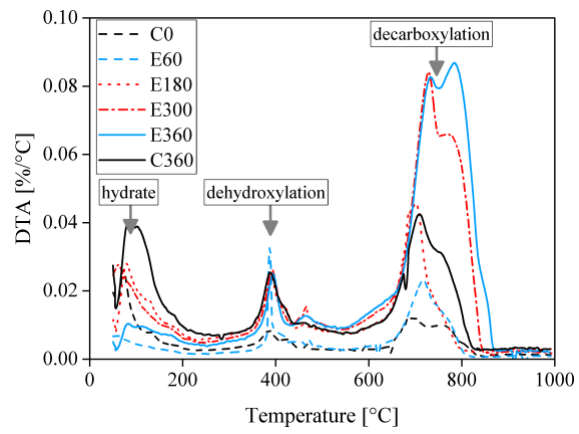


Fig. 4. (a) Freeze-thaw exposure condition; (b) environmental lab condition.

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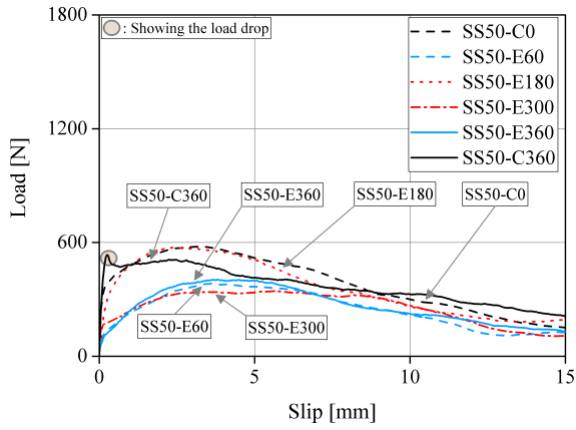
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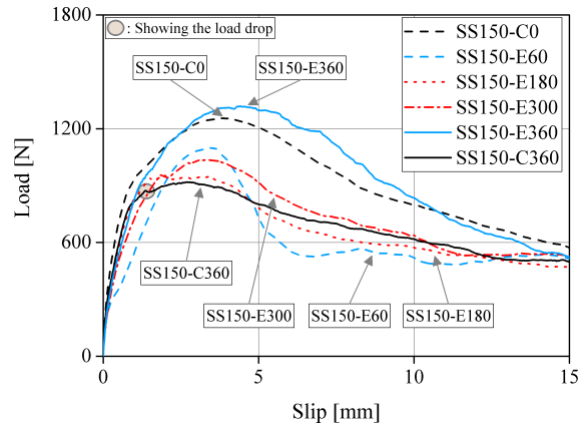


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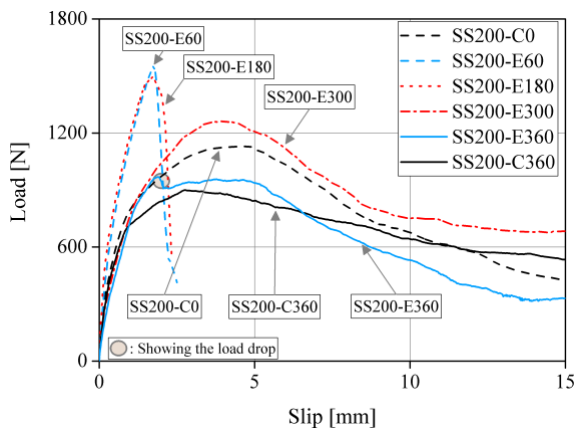
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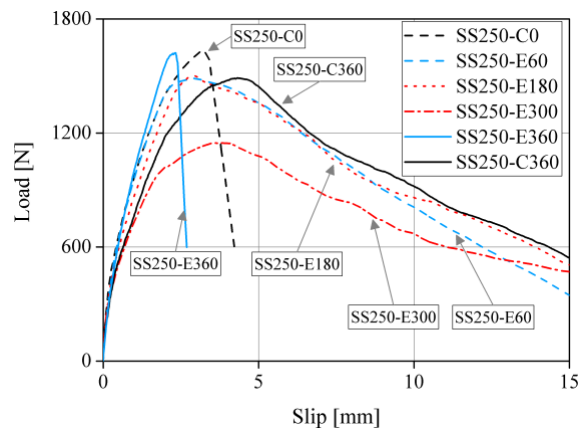
(a)



(b)



(c)



(d)

591 Fig. 6. Average of load-slip response of single steel fibers in different bond lengths: (a) SS50; (b)  
 592 SS150; (c) SS200; (d) SS250.

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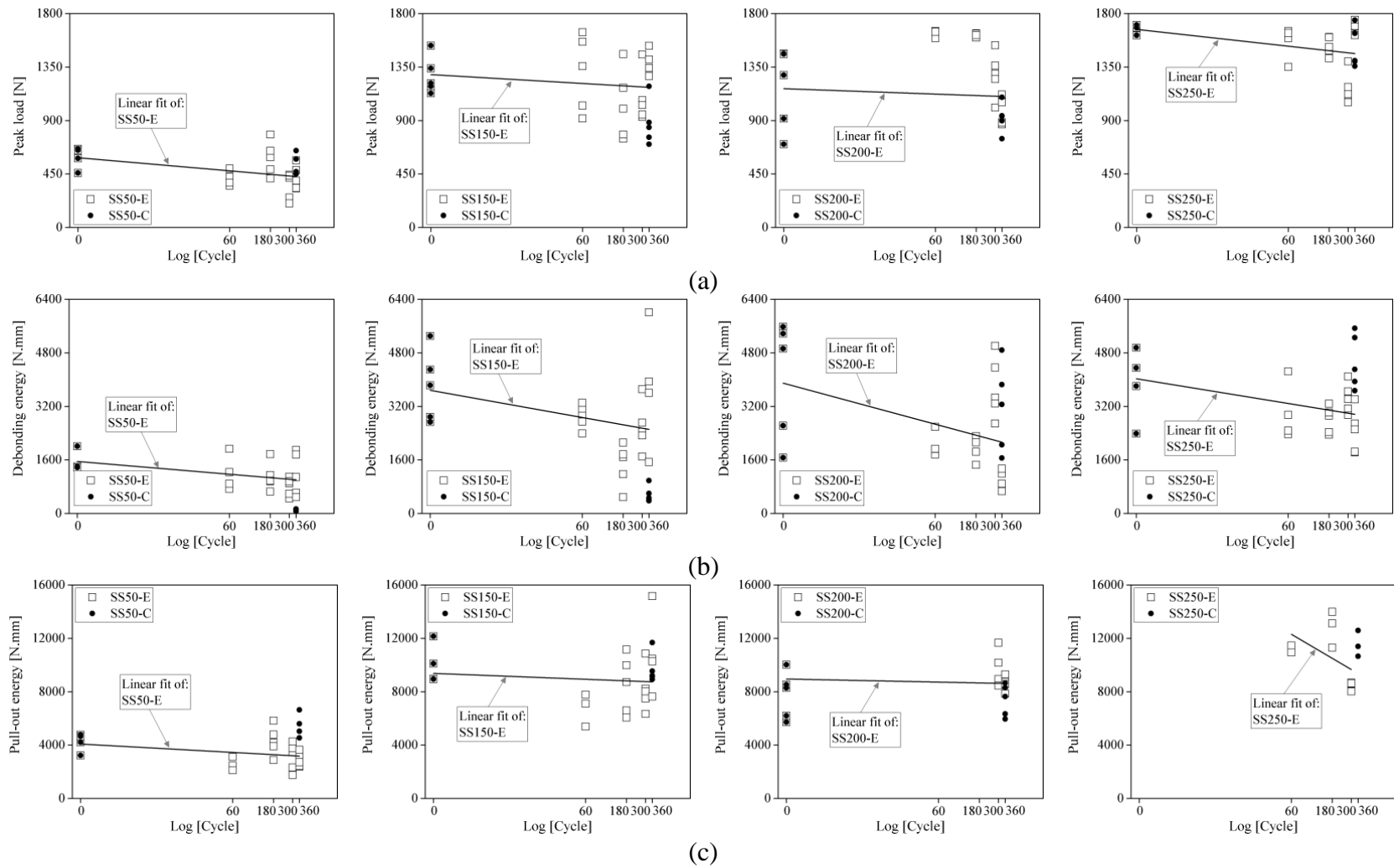
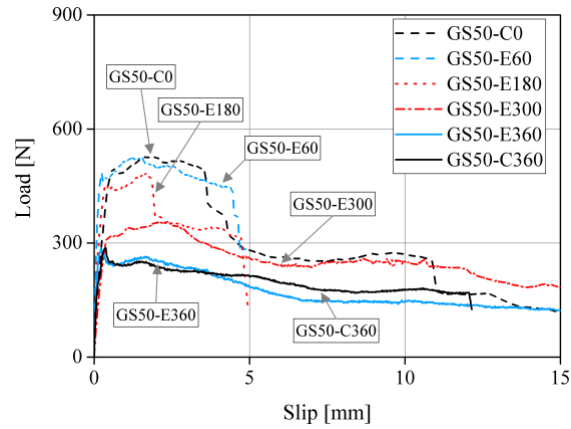
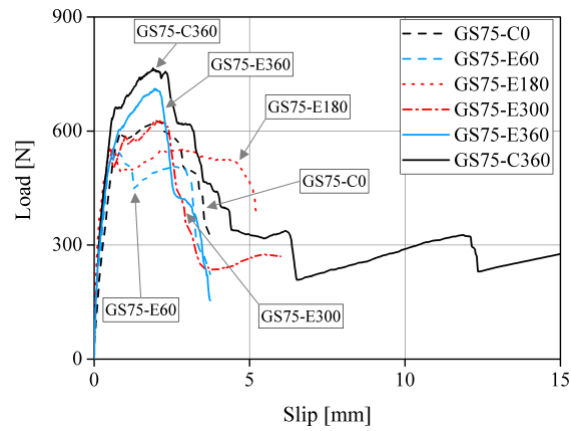


Fig. 7. Pull-out parameters of single steel-based TRM in different bond lengths: (a) peak load; (b) debonding energy; (c) pull-out energy.

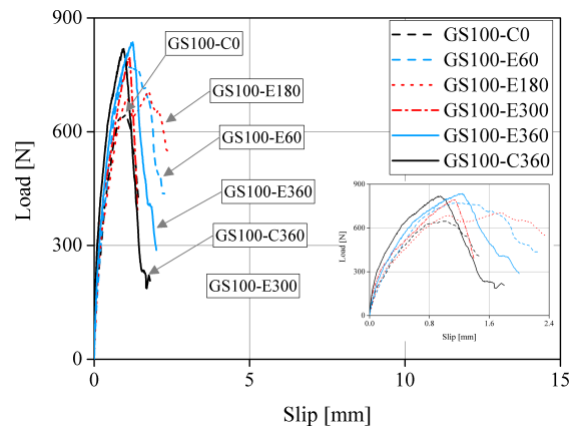
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(a)



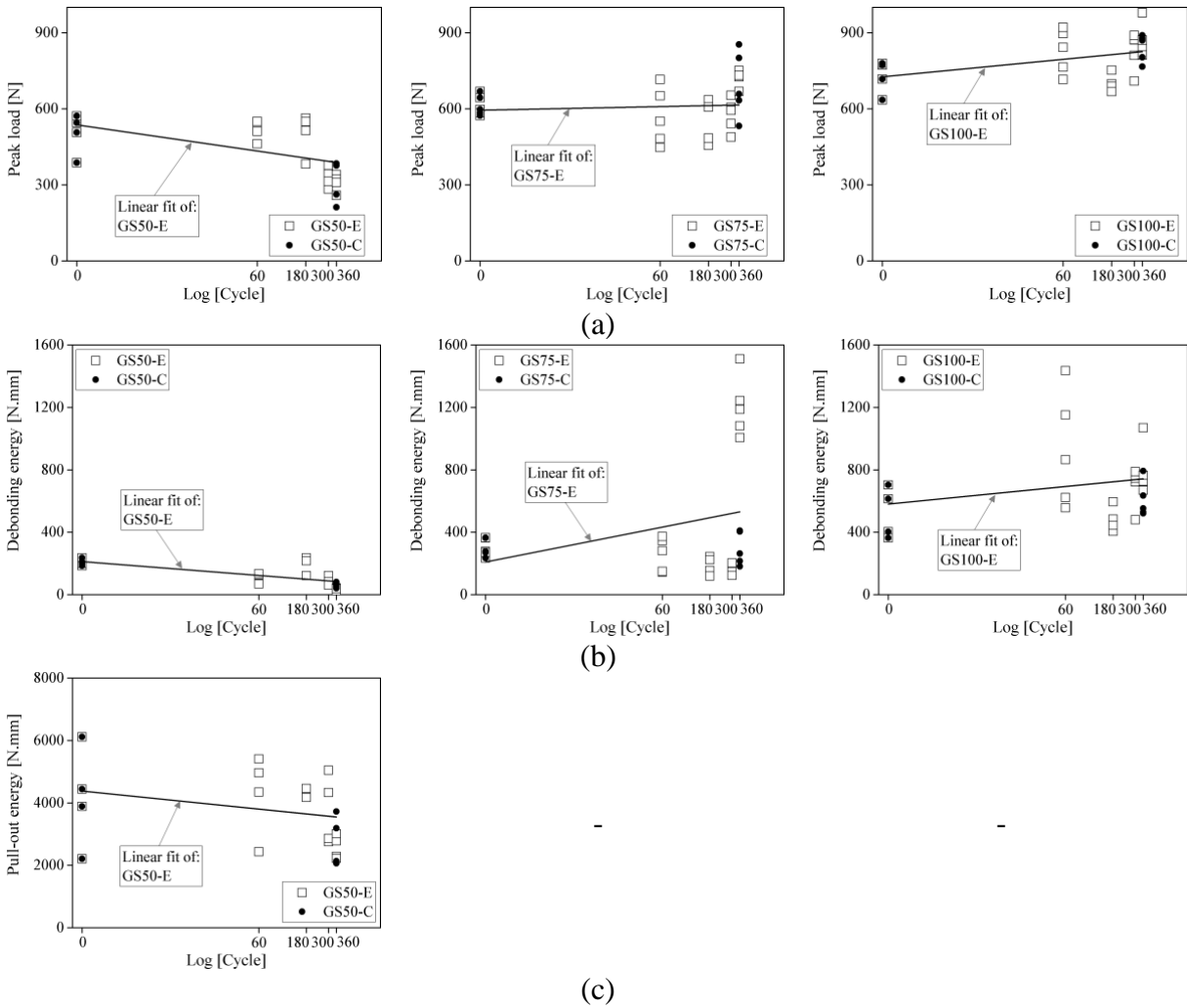
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(c)

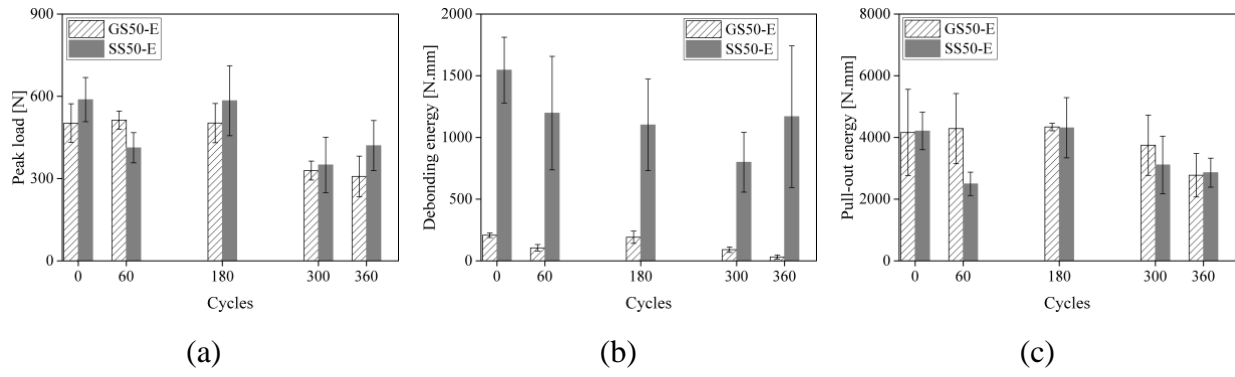
600 Fig. 8. Average of load-slip response of single glass fibers in different bond lengths: (a) GS50;  
 601 (b) GS75; (c) GS100.

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604 Fig. 9. Pull-out parameters of single glass-based TRM in different bond lengths: (a) peak load;  
 605 (b) debonding energy; (c) pull-out energy.

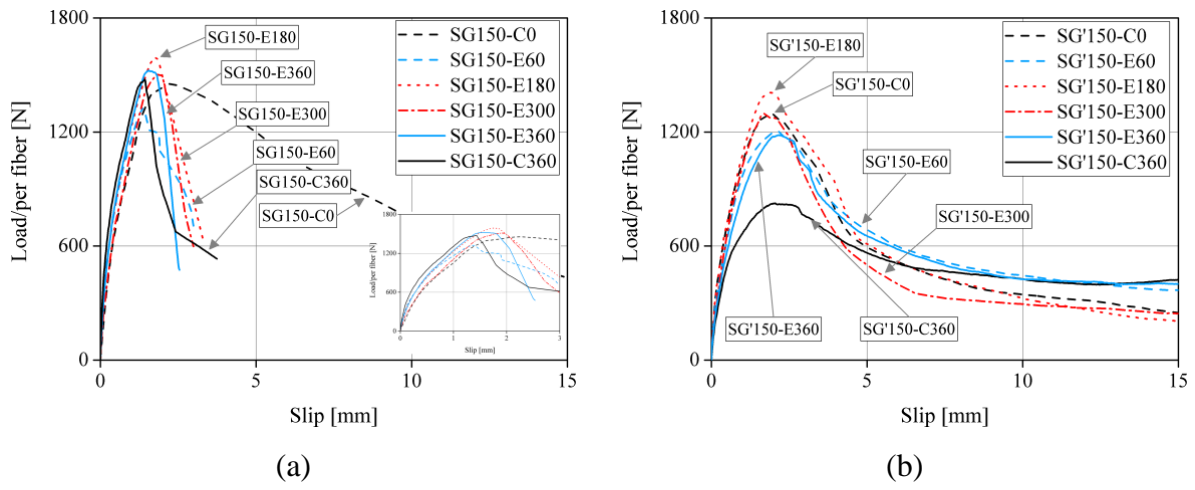




607 Fig. 10. Comparison between pull-out parameters of glass and steel-based TRM in 50 mm bond  
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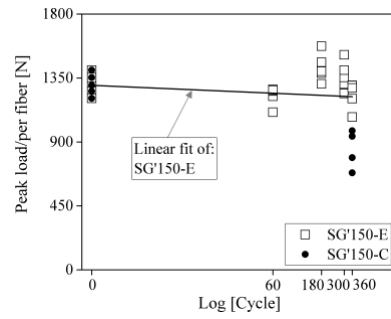
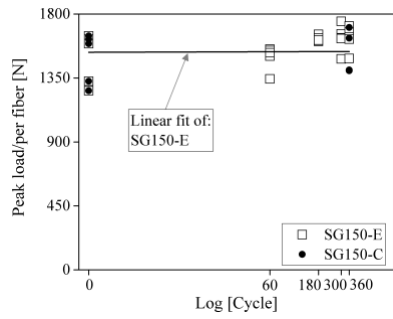
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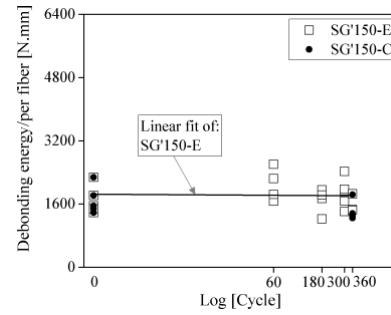
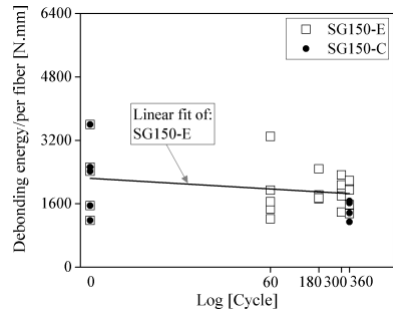
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614 SG150; (b) SG'150.

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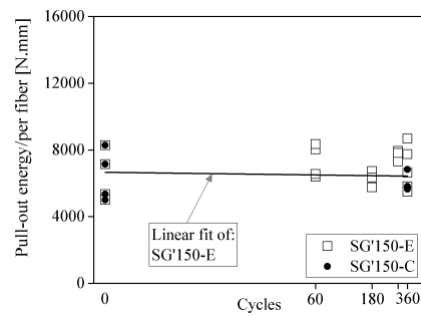
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(a)



(b)

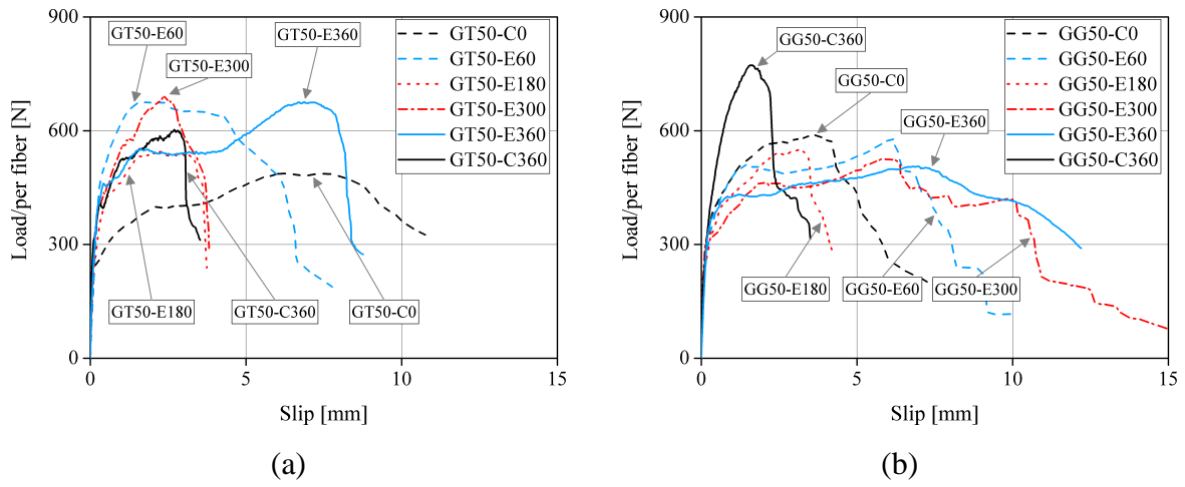


(c)

617 Fig. 12. Pull-out parameters of group steel-based TRM: (a) peak load; (b) debonding energy; (c)  
 618 pull-out energy.

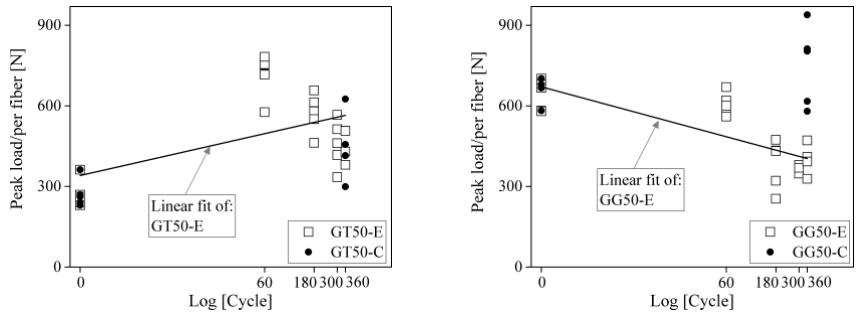
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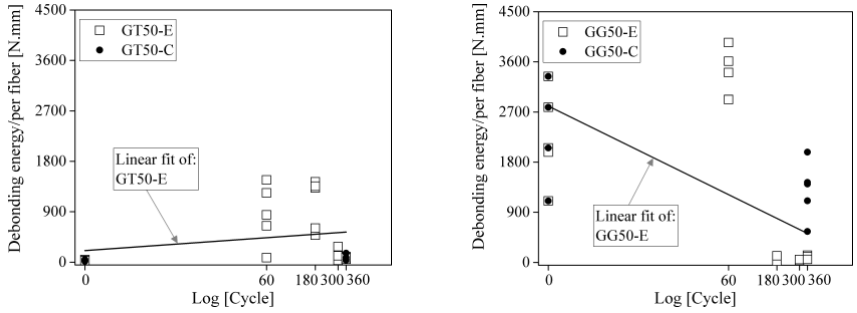


621 Fig. 13. Average of load-slip response of glass-based TRMs with different configurations: (a)  
622 GT50; (b) GG50.

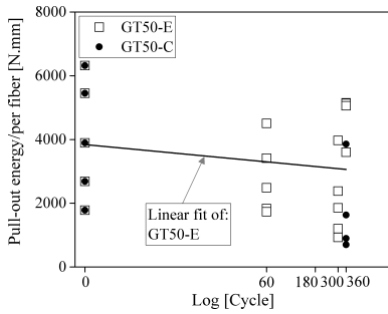
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(a)



(b)



(c)

624 Fig. 14. Pull-out parameters of single+ transverse and group glass-based TRM: (a) peak load; (b)  
 625 debonding energy; (c) pull-out energy.

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